

City University Construction Processes in Geotechnical Engineering

Design of stone columns

Chris Raison BEng MSc CEng MICE MASCE
Raison Foster Associates

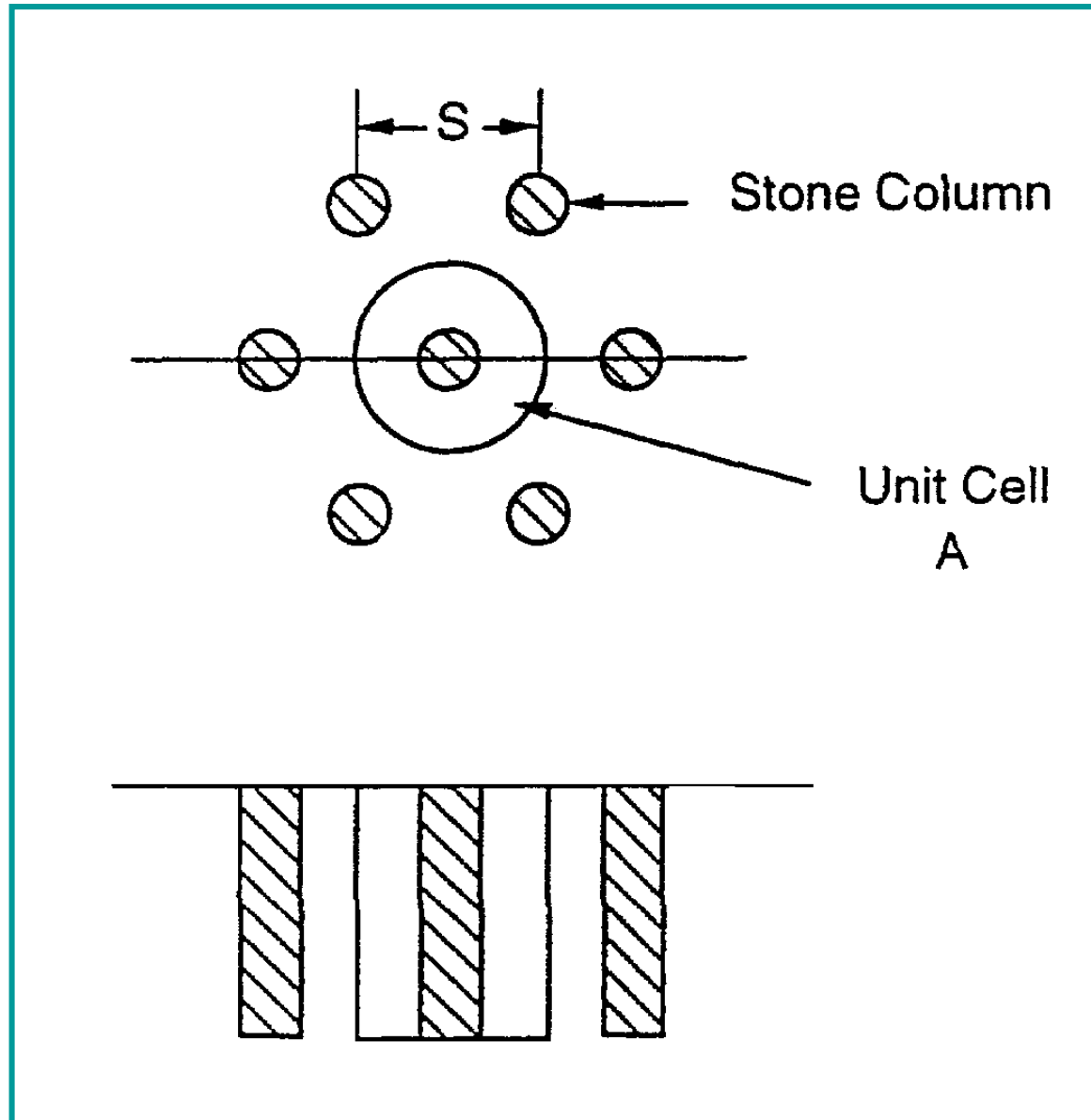
Design Considerations

- Will use of stone columns reduce potential settlements to a tolerable level?
- Is there sufficient bearing capacity provided by stone columns?

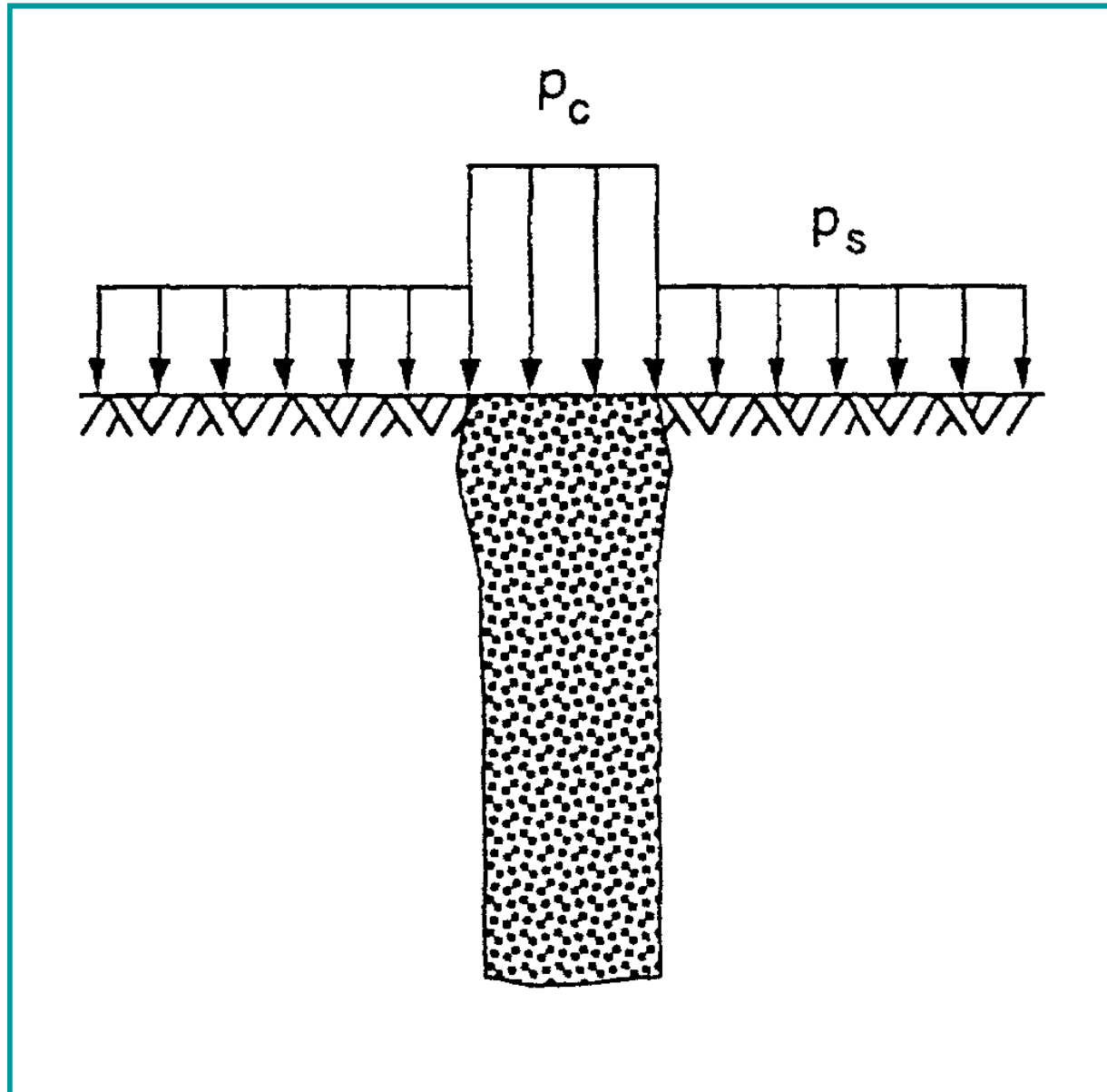
Design Methods

- Reduction in settlement:
 - Baumann & Bauer (1974)
 - Priebe (1976, 1988 & 1995)
- Bearing Capacity of columns:
 - Hughes & Withers (1974)

Concept of Unit Cell Area



Load Share Between Column & Soil



Reduction in Settlement

Baumann & Bauer Method

Canadian Geotechnical Journal, Volume 11, 1974, 509-530

Baumann & Bauer - Equations

$$\frac{P_c}{P_s} = \frac{\left(1 + 2 \frac{E_s}{E_c} K_s \ln \frac{a}{r_0}\right)}{\left(2 \frac{E_s}{E_c} K_c \ln \frac{a}{r_0}\right)}$$

$$P \cdot A = P_c \cdot A_c + P_s \cdot A_s$$

$$a = \sqrt{\frac{A}{\square}}$$

P	bearing stress
r_0	radius of the stone column
A	foundation area
K	earth pressure coefficient
E	stiffness
c	denotes column properties
s	denotes soil properties

Reduction in Settlement

Priebe Method

Die Bautechnik, 53, 1976, 160-162

Die Bautechnik, 65, 1988, 23-26

Ground Engineering, Volume 28, Number 10, December 1995, 31-37

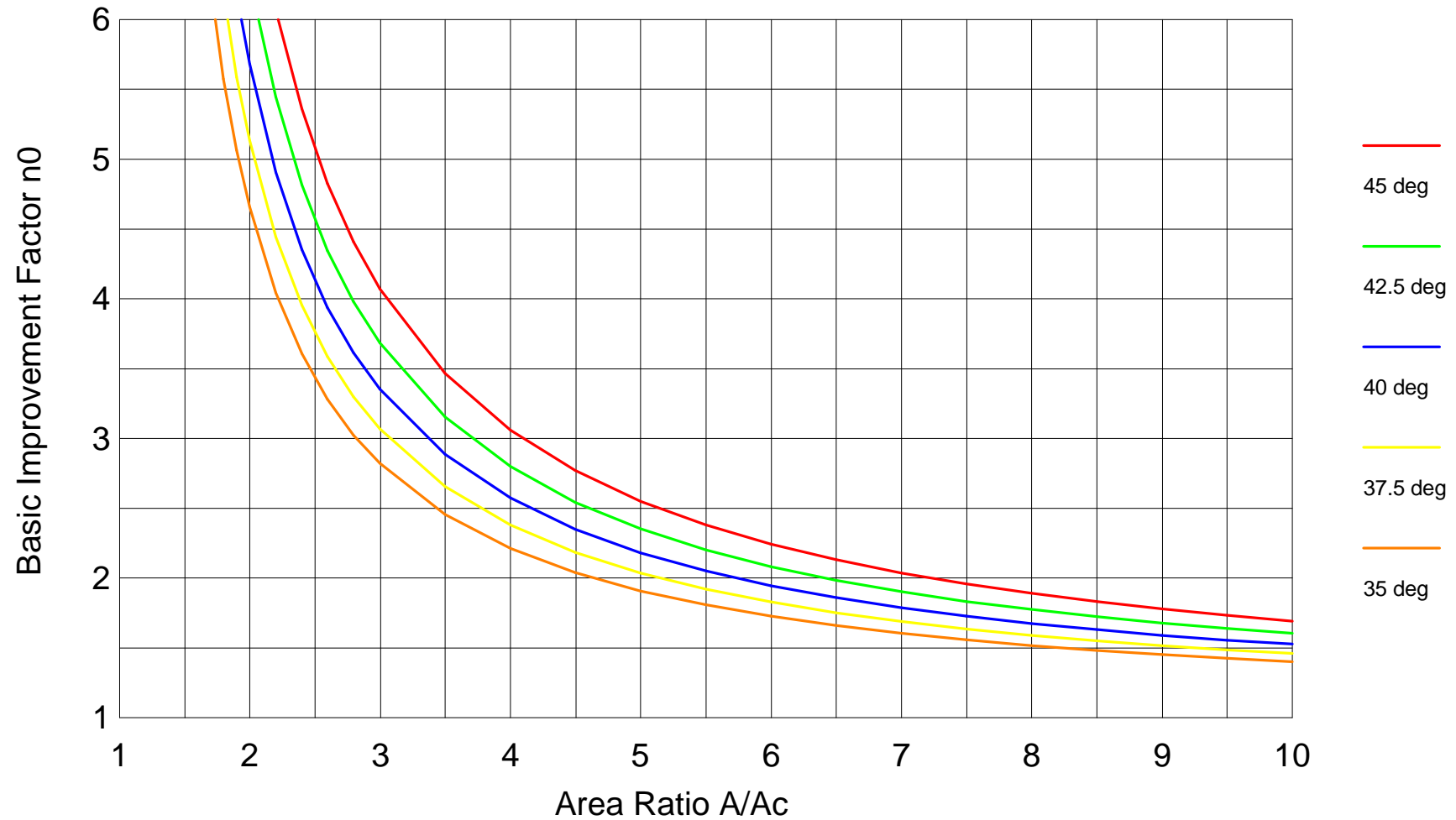
Priebe - Equations

$$n_0 = 1 + \frac{A_c}{A} \left[\frac{0.5 + f\left(v_s, \frac{A_c}{A}\right)}{K_{ac} \cdot f\left(v_s, \frac{A_c}{A}\right)} - 1 \right]$$

$$f\left(v_s, \frac{A_c}{A}\right) = \frac{1 - v_s^2}{1 - v_s - 2v_s^2} \frac{(1 - 2v_s)\left(1 - \frac{A_c}{A}\right)}{1 - 2v_s + \frac{A_c}{A}}$$

n_0	soil improvement
A_c	stone column area
A	foundation area
K_{ac}	active earth pressure coefficient
v_s	Poisson's ratio

Priebe - Basic Improvement n_0



Bearing Capacity of Columns

Hughes & Withers

Ground Engineering, Volume 7, Number 3, May 1974, 42-49

Hughes & Withers

$$P_c = K_{pc}(K_s \cdot \sigma_v' + 4c_u)$$

- P_c stress on top of column
 K_{pc} column passive earth pressure coefficient
 K_s soil earth pressure coefficient
 c_u Undrained shear strength

K_s usually taken as 1 in clays or K_p in granular soils

Conclusions

- Column Parameters
 - ϕ' friction angle for stone column
 - Stiffness E_c
- Soil Parameters
 - ϕ' friction angle or c_u undrained strength for soil
 - Stiffness E_s
 - Poisson's Ratio ν_s
 - Representative earth pressure coefficient K_s

Conclusions

- Column Parameters
 - Little evidence for usual design values
 - Need quality test results
- Soil Parameters
 - Friction angle and stiffness rarely measured
 - Representative K_s needs quality test results